

ชื่อ..... รหัส.....

มหาวิทยาลัยสงขลานครินทร์

คณะวิศวกรรมศาสตร์

การสอบปลายภาค ประจำภาคการศึกษาที่ 2

ปีการศึกษา 2547

วันที่ 25 กุมภาพันธ์ 2548

เวลา 13:30-15:30 น.

วิชา 215-612 ระเบียนวิธีไฟไนท์เอลิเมนท์ (Finite Element Method) ห้อง A201

คำสั่ง

- ไม่อนุญาตให้นำหนังสือหรือเอกสารอื่นได้เข้าห้องสอบ
- อนุญาตให้ใช้เครื่องคิดเลขได้ทุกรุ่น
- ใช้ดินสองหรือปากกาทำข้อสอบก็ได้
- ใช้เวลาทำ 2 ชั่วโมง

ทุจริตในการสอบ โทษต่ำสุด คือ พักการเรียน 1 ภาคการศึกษา และปรับตกในรายวิชาที่ทุจริต

FINAL EXAM:

ข้อสอบมีจำนวน 4 ข้อ ให้ทำทุกข้อ

ข้อ 1. _____ (20 คะแนน)

ข้อ 2. _____ (40 คะแนน)

ข้อ 3. _____ (20 คะแนน)

ข้อ 4. _____ (20 คะแนน)

รวม _____ (100 คะแนน)

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1. (20 points)

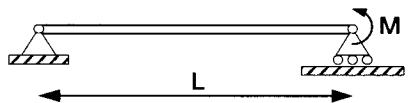
During the finite element course we developed several elements. Fill out the table below for these elements.

ELEMENT	List of Degrees of Freedom	Displacement function $f(x)$ or $f(x,y)$	Sketch of Element (Label DOF)
SPAR in 2D space at angle to x-axis			
Beam along x-axis			
Frame at angle to x-axis			
2D Solid-CST			
2D Solid-LST			

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2. (40 points)

For a **PINNED-GUIDED** beam of length L, with a moment on the Right End, (at $x=L$), use the finite element formulation to solve for the **equation of deflection as a function of x in the beam.**



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3. (20 points)

Describe the concepts of **plane stress** and **plane strain** in detail.

a) Plane Stress (10 points)

b) Plane Strain (10 points)

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4. (20 points)

Use Gaussian quadrature with three Gauss points to evaluate the following integral:

$$T = \int_{-1}^1 \frac{\cos(s)}{1+s^2} ds$$

ATTACHMENT

→ 2D SPAR (along x-axis) axial deflection

$$\text{Stiffness Matrix } [K] = \frac{AE}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad \begin{bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \end{bmatrix}$$

→ 2D Bar (at angle θ to x-axis)

$$C = \cos\theta$$

$$S = \sin\theta$$

$$\text{Stiffness Matrix } [K] = \frac{AE}{L} \begin{bmatrix} C^2 & CS & -C^2 & -CS \\ CS & S^2 & -CS & -S^2 \\ -C^2 & -CS & C^2 & CS \\ -CS & -S^2 & CS & S^2 \end{bmatrix} \quad \begin{bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \end{bmatrix}$$

→ Transformation of a vector from global d to local \hat{d}

(Local x-axis rotated counterclockwise by angle θ from global x-axis)

$$\begin{bmatrix} \hat{d}_x \\ \hat{d}_y \end{bmatrix} = \begin{bmatrix} C & S \\ -S & C \end{bmatrix} \begin{bmatrix} d_x \\ d_y \end{bmatrix}$$

→ 2D Beam (lateral deflection)

$$\text{Stiffness Matrix } [K] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix} \quad \begin{bmatrix} d_{1y} \\ \phi_1 \\ d_{2y} \\ \phi_2 \end{bmatrix}$$

Shape Function of beam element of length L in bending

$$\begin{aligned} v(x) = & [(2/L^3)(d_{1y} - d_{2y}) + (1/L^2)(\phi_1 + \phi_2)]x^3 \\ & + [-(3/L^2)(d_{1y} - d_{2y}) - (1/L)(2\phi_1 + \phi_2)]x^2 \\ & + \phi_1 x + d_{1y} \end{aligned}$$

→ Stiffness Matrix for Frame Element at angle θ to x-axis is

$$[K] = \frac{E}{L} \begin{bmatrix} AC^2 + DS^2 & [A - D]CS & -6IS/L & -[AC^2 + DS^2] & -[A - D]CS & -6IS/L \\ AS^2 + DC^2 & 6IC/L & -[A - D]CS & -[AS^2 + DC^2] & 6IC/L & \\ 4I & 6IS/L & -6IC/L & 2I & & \\ & AC^2 + DS^2 & [A - D]CS & 6IS/L & & \\ \text{Symmetry} & & AS^2 + DC^2 & -6IC/L & & \\ & & & 4I & & \end{bmatrix} \begin{bmatrix} d_{1x} \\ d_{1y} \\ \phi_1 \\ d_{2x} \\ d_{2y} \\ \phi_2 \end{bmatrix}$$

where $D=12I/L^2$ $C=\cos\theta$ $S=\sin\theta$

work-equivalent forces

$$\int w(x)v(x)dx = m_1\phi_1 + m_2\phi_2 + f_{1y}d_{1y} + f_{2y}d_{2y}$$

Types of Elements:

NAME	TYPE	DOF	REAL CONTANTS
LINK1	2D SPAR	UX, UY	A
BEAM3	2D BEAM	UX, UY, ROTZ	A, I, TK
BEAM4	3D BEAM	UX, UY, UZ, ROTX, ROTY, ROTZ	A, IZ, IY, IZ, TKY, TKZ
LINK8	3D SPAR	UX, UY, UZ	A
PIPE16	3D PIPE	UX, UY, UZ, ROTX, ROTY, ROTZ	OD, TK
PLANE42	2D SOLID	UX, UY (PLANE STRESS, PLANE STRAIN, AXISSYMMETRIC)	
SOLID45	3D SOLID	UX, UY, UZ	
SHELL63	3D PLATE/SHELL	UX, UY, UZ, ROTX, ROTY, ROTZ	TK

Pascal triangle

$$\begin{array}{ccccccc} & & & 1 & & & \\ & & x & & y & & \\ & x^2 & & xy & & y^2 & \\ x^3 & & x^2y & & xy^2 & & y^3 \end{array}$$

Table D-1 Equivalent joint forces f_0 for different types of loads

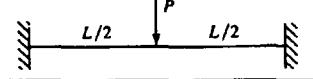
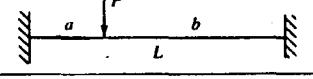
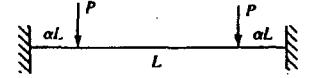
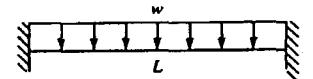
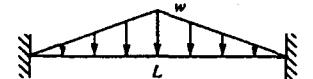
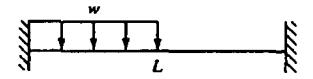
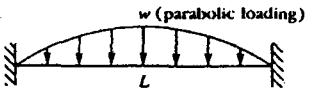
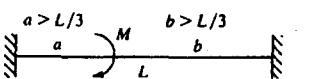
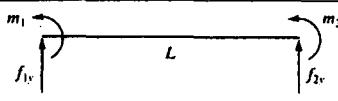
	f_{1y}	m_1	Loading case	f_{2y}	m_2
1.	$\frac{-P}{2}$	$\frac{-PL}{8}$		$\frac{-P}{2}$	$\frac{PL}{8}$
2.	$\frac{-Pb^2(L+2a)}{L^3}$	$\frac{-Pab^2}{L^2}$		$\frac{-Pa^2(L+2b)}{L^3}$	$\frac{Pa^2b}{L^2}$
3.	$-P$	$-\alpha(1-\alpha)PL$		$-P$	$\alpha(1-\alpha)PL$
4.	$\frac{-wL}{2}$	$\frac{-wL^2}{12}$		$\frac{-wL}{2}$	$\frac{wL^2}{12}$
5.	$\frac{-7wL}{20}$	$-\frac{-wL^2}{20}$		$\frac{-3wL}{20}$	$\frac{wL^2}{30}$
6.	$\frac{-wL}{4}$	$\frac{-5wL^2}{96}$		$\frac{-wL}{4}$	$\frac{5wL^2}{96}$
7.	$\frac{-13wL}{32}$	$\frac{-11wL^2}{192}$		$\frac{-3wL}{32}$	$\frac{5wL^2}{192}$

Table D-1 (Continued)

	f_{1y}	m_1	Loading case	f_{2y}	m_2
8.	$\frac{-wL}{3}$	$\frac{-wL^2}{15}$		$\frac{-wL}{3}$	$\frac{wL^2}{15}$
9.	$\frac{-M(a^2 + b^2 - 4ab + L^2)}{L^3}$	$\frac{Mb(2a-b)}{L^2}$		$\frac{M(a^2 + b^2 - 4ab + L^2)}{L^3}$	$\frac{Ma(2b-a)}{L^2}$
 Positive nodal force conventions					

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Table for Gauss points for integration from minus one to one $\int_{-1}^1 y(x)dx = \sum_{i=1}^n W_i y_i$:

Number of Points	Locations, x_i	Associated Weights, W_i
1	$x_1=0.000$	2.000
2	$x_1=+0.57735026918962$ $x_2=-0.57735026918962$	1.000 1.000
3	$x_1=+0.77459666924148$ $x_2=0.000$ $x_3=-0.77459666924148$	5/9=0.555... 8/9=0.888... 5/9=0.555...
4	$x_1=+0.8611363116$ $x_2=+0.3399810436$ $x_3=-0.3399810436$ $x_4=-0.8611363116$	0.3478548451 0.6521451549 0.6521451549 0.3478548451